ENGINEERING PROPERTIES OF SEDIMENTS IN THE VICINITY OF GUIDE SEAMOUNT

by

Jerome Ronald Heck
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September 1970

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Engineering Properties of Sediments
in the Vicinity of Guide Seamount

by

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requirements for the degree of

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ABSTRACT

The sophisticated vane shear apparatus for determining the shear strength of deep ocean sediment cores was modified so as to be portable and versatile for use in a laboratory and on board a ship. The apparatus utilizes a torque transducer that is insensitive to temperature changes or orientation and capable of measuring torque over the entire range of shear strength encountered in marine sediments. Shear strength measurements can be made with minimum disturbance to the sediment sample by testing directly in the core liner. The apparatus was used to determine shear strength of ten deep ocean cores from the Guide Seamount region, located about 70 miles west of the central California coast. The study also included the determination of other engineering properties of the sediment cores.
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ACKNOWLEDGEMENTS

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I. INTRODUCTION

A. GENERAL

The rapid advance in science and technology within the past several years has set the stage for more complete undersea exploration. It is apparent that there will be an increased use of both shallow and deep marine waters for various purposes. Because underwater structures will be supported by or penetrate into the sea floor, a knowledge of the engineering properties of the sea floor is required. Bearing capacity, breakout forces, trafficability, and slope stability are some of the important areas which are directly or indirectly related to the shear strength of sediments.

The present techniques for sea floor sediment investigation are far from being perfected. The problems of how to obtain an undisturbed sample or make in-situ measurements still have to be overcome.

Analyses can be made on slightly disturbed samples obtained with coring tools. Some of the present methods used to determine the strength parameters of deep sea sediments include unconfined compression, triaxial shear, direct shear, vane shear, and cone penetrometer tests. According to Richards (1961) and Smith (1962), the disturbance of a sample is minimized by employing the vane shear test. Because the sample does not have to be removed from the core liner prior to testing, use of the vane shear permits working with sediments of high water content. The vane shear technique of obtaining shear strength is open to some criticism, but the measured results are frequently found to be of the same order of magnitude as those obtained by more standard methods. Some of the marine sediments can only be handled in this manner as a result of their low consistency.
B. SCOPE OF STUDY

A versatile vane-shear apparatus was developed by Minugh (1970) utilizing a constant speed motor train and transducer to measure the amount of torque on the vanes at the time of sediment failure. In that the components of this apparatus were large and heavy, a more lightweight and portable apparatus would be desirable. Modifications were made to the apparatus to facilitate a more positively controlled height adjustment system. Other components were pared down in size and weight to meet the portability requirement.

Ten deep sea sediment cores were then taken in the vicinity of Guide Seamount and the following engineering properties were studied:

a. Vane Shear Strength
b. Remolded Strength
c. Sensitivity
d. Bulk Wet Density
e. Water Content
f. Specific Gravity of Solids (computed from the bulk wet density and water content)
g. Dry Density
h. Void Ratio
i. Porosity

The newly designed vane shear apparatus was tested on board ship at sea during this program and the results obtained were compared with those found in laboratory tests.

C. SUMMARY OF PREVIOUS WORK

1. Vane Shear Test Apparatus

Present day vane shear test devices are adaptations of the early vane borers. The vane borer was developed simultaneously in Sweden by John Olsson in 1928 and in Germany in 1929, according to
Osterberg (1957). Cadling and Odenstad (1950) then described a method of obtaining the shear strength of clays in-situ by using a vane device.

The shear strength of the soil was considered as related to the maximum torque developed from vane rotation by the following formula, derivation of which appears in Appendix A:

\[ s = \frac{T}{(2\pi r^2 h + 4/3 \pi r^3)} \]

where

- \( s \) = shear strength
- \( T \) = maximum torsional moment required to produce shear
- \( r \) = radius of vane
- \( h \) = height of vane

According to Carlson (1948), this formula assumes that the surface of rupture is a circular cylinder surrounding the vane and at the moment of maximum torque the shear stress is fully developed and uniform over the entire surface, including the ends of the cylinder. The above assumption appears valid and is consistent with the results of tests conducted by Inderbitzen (1969). A more detailed description of the vane shear test and its theory is presented by Skempton (1948) and Skempton and Bishop (1950).

Wilson (1963) reports that for cohesive soils (clays) the vane shear test has been found to be a reliable means of determining shear strength, especially in the case of sensitive soils and ones of lower shear strength. Hayen and Cohen (1967) note that the major limitation of the vane shear device is that it measures mainly cohesion. Whenever sandy or silty sediments are present, therefore, the shear strength
measurements with the vane provide a measure of the angle of internal friction in that the overburden pressure increases with sediment depth and a reading of vane shear strength at different depths gives a stress envelope from which the angle of friction can be derived. This procedure assumes a consistent sediment type through the depth of measurement. The derivation of cohesion and angle of friction is shown by Hayen and Cohen (1967); however, the effects of consolidation are not taken into account with this approach.

There are several varieties of vane shear testing equipment currently in use to determine the shear strength of marine sediments. The devices most widely used are:

a. Wykeham Farrance Vane Shear Apparatus, manufactured in England.
b. NCEL Vane Shear Device, designed by Smith (1962) at the Naval Civil Engineering Laboratory.
c. IIT Vane Shear Test Apparatus by Vey and Nelson (1966) at the Illinois Institute of Technology Research Institute.
d. Diver-Held Vane Shear Apparatus, developed by Dill and Moore (1965).

In comparison to the above, the vane shear apparatus designed by Minugh (1970) is the most sophisticated. The apparatus is made up of a constant speed electric motor, power supply and signal conditioning unit, and the output from the transducer to the recorder gives a continuous record of the amount of torque applied to the vane.

2. **Guide Seamount**

There have been limited studies conducted in the vicinity of Guide Seamount. The seamount is located about seventy miles off the coast of California, between Monterey and San Francisco at 37°-01.5' North Latitude and 123°-20.5' West Longitude, as shown in Figure 1. The seamount is approximately 13 kilometers long, 10 kilometers wide, and
Figure 1: The location of Guide Seamount with respect to the central California coast.
1000 meters high. The study of submarine topography off the California coast, conducted by Shepard and Emery (1941), suggests that Guide Seamount is possibly a submarine volcano. It exhibits a northeasterly trend and has more than one summit. There is also evidence of the presence of small valleys on the sides. Its proximity to the base of the continental slope suggests that the two are interrelated, according to Uchupi and Emery (1961). The base of the slope represents a transition zone from the continental mass to the ocean floor and is therefore possibly an unstable region favorable for igneous activity.

Hanna (1952) states that only mud was obtained from Guide Seamount in a 15-inch dredge. Uchupi and Emery (1961) report that basalt pebbles and cobbles recovered from the seamount were found to be well-rounded, black to greyish-black in color, and also sometimes phryritic and vesicular. The presence of pyroclastic material and the vesicular nature of the lava fragments suggested to MacDonald (1954) that the volcanic eruptions did not take place at their present water depth. The rounded nature of most of the lava fragments present on top of the seamount also indicates the possibility that the crest of the submarine volcano was once exposed to wave action. Perhaps the sinking of the cone to its present depth may have been isostatic in response to its own weight. The age of the volcanism is not known, but the fact that the crest of the seamount is not buried under a blanket of sediments suggests a Late Tertiary age, possibly Miocene or younger.

No work has been done on the engineering properties of the marine sediments in the Guide Seamount region prior to this investigation. Most of the earlier investigations were either geologic in nature or sea floor contour studies.
II. DESIGN CONSIDERATIONS

A. PORTABLE VANE SHEAR APPARATUS

The existing vane shear apparatus is designed so as to be configured for laboratory or shipboard use. It was found, however, that it was too heavy and bulky to be considered as truly a portable device. Also, the rack and pinion vane height adjustment was not positively controlled and the device would tend to lower itself due to the weight of the motor and transducer. The apparatus consisted of the following components:

a. torque transducer 
b. power supply and signal conditioning unit 
c. bracket arm 
d. swivel assembly 
e. rack and pinion assembly 
f. motor and motor mount 
g. calibration stand and wheel 
h. recorder

B. DESIGN CRITERIA

The design criteria used to improve the existing vane shear apparatus were as follows:

a. The apparatus would incorporate the original motor and torque transducer.
b. The weight and size of all the supporting components were to be reduced.
c. Design a positive vane height adjustment mechanism that could not free fall.
d. Design the apparatus to be portable with a carrying case.
e. Design the apparatus for various configurations of use, such as:
   1. A table stand arrangement that could be used in any orientation.
   2. A wall mounting system where the apparatus could be used in various orientations and easily removed.
   3. A method of using the apparatus for testing directly on a core liner.
4. The newly designed apparatus was adapted to the original mounting system to give added versatility.

f. The calibration system was incorporated into the carrying case to eliminate extra components.

g. Insure that there was laboratory and shipboard compatability of the apparatus.
The Naval Postgraduate School (NPS) Vane Shear Apparatus is basically the same instrument as designed by Minugh (1970) for laboratory and shipboard use. Several modifications have been included to improve the design of versatility and portability. The complete apparatus consists of the following components which are described in detail in the following section:

a. Constant speed electric motor, motor mount and housing
b. Torque transducer
c. Power supply and signal conditioning unit
d. Height adjustment mechanism
e. Core holding bracket
f. Wall mounting brackets
g. Table stand
h. Ball joint assembly
i. Modified swivel assembly to fit the original stand designed by Minugh
j. Set of various size vanes
k. Strip chart recorder
l. Carrying case and calibration system

1. Motor, Mount and Housing

The single phase, synchronous, heavy duty motor Model EA - H used in this device is manufactured by Hurst Manufacturing Corporation, Princeton, Indiana. It is rated at 150 inch-ounces of torque at one revolution per minute and requires 115 volts AC 60 cycle power. The output shaft rotates at one revolution per hour in a counter-clockwise direction. The reduction gears used are contained in a sealed unit and require no lubrication. The overall dimensions and characteristics are
shown in Fig. 2, supplied by the Hurst Manufacturing Corporation.

The motor is connected to the height adjustment mechanism by means of the aluminum mounting shown in Fig. 3 and Fig. 4. The plastic motor housing that was fabricated to protect the motor is shown on Fig. 3 and also on Fig. 5.

2. Torque Transducer

The torque transducer shown in Fig. 6 is an in-line semiconductor strain gage type, Model A44, manufactured by West Coast Research Corporation of Santa Monica, California. The range is 0-250 inch-ounces, although it may be over-torqued 100 percent without damage. The output of the transducer is 0.269 millivolts/volt excitation/in-ounce, and is linear throughout the entire range. Accuracy of the torque measurement is ±0.1 percent throughout the range. The internal resistance of the transducer is 350 ohms. The temperature response is 0.0045 millivolts/degree Fahrenheit, with 72 degrees Fahrenheit being the calibration temperature. The transducer will measure either clockwise or counter-clockwise torque, the polarity of the output signal indicating the direction. Excitation to the strain gages is a regulated five volt DC signal from the power supply unit. The output is unaffected by the orientation and it may be used horizontally, vertically, or obliquely.

The transducer is joined to the motor assembly by a 1/2" long 1/4"-28 threaded stud. The vanes are screwed into a 1/4"-28 female socket in the transducer. The transducer is connected to the power supply and signal conditioning unit by a four wire conductor cable.

When used for testing, the motor forces the vane to rotate in the sample. Resistance to this rotation provided by the sediment is opposite in direction to the vane rotation direction. This produces
Figure 2: Dimensions of the motor used in the NPS Vane Shear Apparatus
Figure 3: The NPS Vane Shear Apparatus as used when performing laboratory tests.
Figure 4: Detail drawing of the aluminum motor mount for the NPS Vane Shear Apparatus
Figure 5: Detail drawing of the plastic motor housing for the NPS Vane Shear Apparatus.
Figure 6: The torque transducer, vane, and core holding bracket for the NPS Vane Shear Apparatus.
a twisting moment to the torque transducer. The strain gages attached to the inner shaft of the transducer measure the shaft deflection caused by this twisting moment. The output of the strain gages is linear and is directly proportional to the amount of shaft deflection.

3. **Power Supply and Signal Conditioning Unit**

The power supply and signal conditioning unit is a combined transistorized power supply, bridge circuit, and amplifier, as shown in Fig. 3. The power supply provides five volt DC excitation to the strain gages. The output signal from the strain gages produces an imbalance in the bridge circuit proportional to the torque applied to the transducer. This imbalance results in an output which is fed through a variable gain amplifier to the recorder.

The unit is provided with a push button resistive circuit equivalent to a 125 inch-ounce torque and may be used to adjust the amplifier gain. When the "R Cal" button on the back panel is depressed, the signal from the strain gages is interrupted and the electrical equivalent of the 125 inch-ounce torque reading is substituted. Minugh (1970) recommended that a one volt full scale reading be set on the recorder, the "R Cal" button depressed, and the amplifier gain adjusted until the recorder trace reads 0.5 volts. This procedure sets the amplifier at four millivolts per inch-ounce torque. The balance knob is comparable to a "zero adjust" and allows the reference level to be shifted to any desired position.

4. **Height Adjustment Mechanism**

The height adjustment mechanism shown in Fig. 3 and Fig. 7 is constructed from aluminum and consists of the following components:
Figure 7: Detail drawing of the height adjustment mechanism for the NPS Vane Shear Apparatus
b. Threaded shaft with keyway.
c. Height adjusting nut.
d. Three locking nuts.
e. Two key pins.

The motor is fastened to the lower end of the threaded shaft by means of a socket in the motor mount and secured by a locking screw. The motor may also be rotated about the shaft to provide any desired orientation.

The mechanism can be bolted to various mountings so as to permit versatility in use. A means of positive control when lowering the vane into the sample is thus provided, and the unit will remain wherever stopped. The top locking nuts are used to set a desired stopping depth for multiple sample tests and the bottom nut is used to lock the entire assembly to keep it free from any motion. Application of the bottom nut is usually not necessary in the laboratory; however, aboard ship it aids in insuring that the apparatus is rigid.

5. Core Holding Bracket

The core holding bracket shown in Fig. 8 and Fig. 9 can be used in two different ways. One of these is to attach the bracket to the wall through use of a quick-removal wall mount. One of these was constructed of wood for laboratory use and another was constructed of aluminum, as shown in Fig. 3 and Fig. 8 as part of the portability package. The other method of using the core holding bracket is to attach it directly to a core, as shown by Fig. 10. The core must first be made secure in order to support the weight of the apparatus. The quick-release core holder shown in Figs. 6, 8 and 9 was designed to be used in conjunction with plastic core liners of a Ewing corer. The holder is constructed of aluminum and has a sponge rubber lining which absorbs the compressive
Figure 8: The core holding bracket showing the quick-release latch and the quick-removal wall mounting bracket for the NPS Vane Shear Apparatus
Figure 9: Detail drawing of the core holding bracket for the NPS Vane Shear Apparatus
Figure 10: The NPS Vane Shear Apparatus in the core mounted testing configuration
force of the closed casing. None of this force which might well produce disturbance in the sample is applied to the liner. The friction developed is sufficient to hold a core 60 inches long.

6. Wall Mounting Brackets

Three different wall mounting brackets were designed. The wooden bracket shown in Fig. 3 and Fig. 11 was constructed to hold the apparatus during use in the laboratory in conjunction with the core holding bracket. The aluminum bracket shown in Fig. 8 and Fig. 12 was also fabricated to be used in the same manner. The third wall bracket, shown in Fig. 13, is a straight mounting for the height adjustment mechanism. The ball joint assembly can also be mounted between the wall bracket and the height adjustment mechanism so as to give a freedom of orientation.

7. Table Stand

The table stand is made from 5/8 inch aluminum plate, eight inches square, with four 1/4 inch holes drilled near the corners to allow the stand to be bolted to a workbench. Two vertical aluminum rods one inch in diameter and twelve inches long provide a method of separation and attachment between the base plate and the height adjustment mechanism, as illustrated in Fig. 14 and Fig. 15. The vertical rods are offset in the center, both to give better balance to the instrument and to provide additional flexibility.

8. Ball Joint Assembly

The connecting link between the vertical rods and the table stand is the ball joint assembly shown in Figs. 14, 16 and 17. This assembly consists of the locking collar which clamps around the vertical rod, the ball joint unit for varied orientation, and the connecting plate used for attaching to the height adjustment mechanism.
Figure 11: Detail drawing of the wooden wall mounting bracked used in the laboratory with the NPS Vane Shear Apparatus
Figure 12: Detail drawing of the aluminum quick-removal wall mounting bracket for the NPS Vane Shear Apparatus
Figure 13: Detail drawing of the wall mounting plate for the NPS Vane Shear Apparatus
Figure 14: The NPS Vane Shear Apparatus in the table stand testing configuration
Figure 15: Detail drawing of the table stand for the NPS Vane Shear Apparatus
Figure 17: The NPS Vane Shear Apparatus in the table stand configuration depicting oblique testing
9. **Swivel Assembly**

The swivel assembly, as originally designed by Minugh (1970), was modified with the mating plate shown in Fig. 18 to accept the height adjustment mechanism. The swivel assembly shown in Fig. 19 rotates in two planes, whereas the ball joint provides a complete freedom of orientation of the apparatus. This modification of the swivel assembly allows the original stand as designed by Minugh to also be used.

10. **Vanes**

Five vanes were constructed for use with the vane shear apparatus and have the dimensions shown on Fig. 20. Both the vane and the shaft were constructed of stainless steel, and were fabricated independently and then silver-soldered together. The top of the shaft is designed to thread into the base of the transducer unit.

The denominator in the shear strength equation,

\[ \frac{\pi d^2 h}{2} + \frac{\pi d^3}{6} \]

where \( d \) is the diameter and \( h \) is the height of the vane, is a constant for each vane size. Referring to this denominator as the Vane Factor, a listing of each size vane and its appropriate Factor is as follows:

<table>
<thead>
<tr>
<th>Vane Dimensions (inches)</th>
<th>( d )</th>
<th>( h )</th>
<th>Vane Factor ((\text{in}^3))</th>
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<tbody>
<tr>
<td>Height 1</td>
<td>1/2</td>
<td>1/2</td>
<td>0.45813</td>
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<tr>
<td>Height 1/2</td>
<td>1/2</td>
<td>1/2</td>
<td>0.61780</td>
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<tr>
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<td>1/2</td>
<td>1.30899</td>
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<tr>
<td>Height 1</td>
<td>1</td>
<td>1</td>
<td>2.09438</td>
</tr>
<tr>
<td>Height 1-1/2</td>
<td>1</td>
<td>1</td>
<td>2.87978</td>
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Figure 18: Detail drawing of the swivel assembly mating plate for the NPS Vane Shear Apparatus
Figure 19: The NPS Vane Shear Apparatus mounted on the swivel assembly
### VANE DIMENSIONS (in)

<table>
<thead>
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<th>D</th>
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<th>L₁</th>
<th>L₂</th>
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<td>1-1/2</td>
<td>2-1/4</td>
<td>4-1/4</td>
</tr>
</tbody>
</table>

Figure 20: Detail drawing of the vanes for the NPS Vane Shear Apparatus
From the equation above, the shear strength in ounces per square inch is obtained by dividing the maximum torque required to shear the sediment in inch-ounces by the appropriate Vane Factor. Most sediment shear strengths are presently reported in units of pounds per square inch; hence, a computer program was written to convert directly from millivolt readings obtained from the recorder to shear strength in pounds per square inch. The millivolts were incremented by tenths from 0 to 500, generating the tabulation included as Appendix E. In order to use this tabulation, the amplifier gain must be adjusted as previously described so that four millivolts equals one inch-ounce of torque. The shear strength, in pounds per square inch units, is then obtained from the following relationships:

\[ 4 \text{ Millivolts} = 1 \text{ inch-ounce Torque} \]

\[ \text{Torque (in-oz)} = \frac{\text{Recorder Reading (millivolts)}}{4} \]

To obtain the shear strength in inch-ounces, enter the above torque value in the shear strength equation, where the Vane Factor is given by \( \pi d^2 n/2 + d^3/6 \) for a particular size vane:

\[ \frac{\text{Shear Strength (oz/in}^2\text{)}}{\text{Vane Factor (in}^3\text{)}} = \frac{\text{Torque (in-oz)}}{\text{Vane Factor (in}^3\text{)}} \]

The conversion of shear strength in ounces per square inch to pounds per square inch is:

\[ \text{Shear Strength (psi)} = \text{Shear Strength (oz/in)} \times \frac{1}{16} \text{ (lb/oz)} \]

Combining terms results in the simple relationship below for determining shear strength in pounds per square inch from the recorder reading in millivolts and a given vane factor:

\[ \text{Shear Strength (psi)} = \frac{\text{Recorder Reading (millivolts)}}{64 \times \text{Vane Factor}} \]
11. **Recorder**

Any quality recorder may be used in conjunction with the vane shear apparatus. The Hewlett-Packard 680 Strip Chart Recorder shown in Fig. 3 was used for all the tests of the present investigation.

12. **Carrying Case and Calibration System**

The carrying case shown in Fig. 21 was designed to house all components of the system except for the recorder. Mounting brackets and methods of securing all components to ensure that they are well protected were built into the carrying case. The calibration system has been incorporated in the carrying case. The system is arranged by attaching two arms with small pulleys to the top of the case, as is illustrated by Fig. 22. A calibration wheel is then attached to the other end of the transducer so as to permit a known torsional moment be ap be applied. Known weights are then tied to the end of a piece of lightweight fishing line and attached to the calibration wheel. These lines are passed over the small pulleys and the weights are allowed to hang free so as to apply a known torsional moment. The pulleys are ball-bearing mounted to eliminate friction. The output signal produced from this should now correspond to the reading of the known torque. If the amplifier gain has been properly adjusted by means of the "R Cal" feature as previously described to indicate four millivolts per inch-ounce of torque and if each weight suspended is five ounces, the total torque applied is ten inch-ounces. The output reading from the transducer should be 40 millivolts. A calibration curve was plotted using various weights to verify the linearity of the transducer when adjusted by means of the "R Cal" feature.
Figure 21: The NPS Vane Shear Apparatus components as stored in the carrying case
Experience has demonstrated that adjustment by the "R Cal" of the power supply and signal conditioning unit is very accurate. The calibration procedure is not necessary in the course of normal operation and need only be used in the event there is reason to believe that the resistive value of the "R Cal" circuit has changed.

B. TYPICAL VANE SHEAR RECORD

Figure 23 represents a typical continuous record for a cohesive marine sample tested with the NPS Vane Shear Apparatus. The curves shown are from the 3-6 inch interval of core 8 H. The lower curve is the original undisturbed sample and the upper curve indicates the loss of strength in the remolded sample. Both were tested using a 1/2 inch diameter by one inch high vane and with a ten millivolt scale setting on the strip chart recorder. The resulting shear strength was 0.276 pounds per square inch for the original and 0.068 pounds per square inch for the remolded sample. These values can be obtained by using the shear strength formula as shown previously or by utilizing the tables of Appendix E. According to Smith (1962), when using the NCEL Vane Shear Device, marine materials exhibit a flat crest on the curve from vane rotation angles of 27 to 38 degrees, and it is presumed that this interval represents failure of the cylindrical segment of soil. As evidenced by Fig. 23, the maximum reading occurs at about 18 degrees of vane rotation. Also, Fig. 23 shows that the curves do exhibit a flattened region; however, experience has demonstrated that this area usually occurs around 15 to 24 degrees of vane rotation with the NPS Vane Shear Apparatus. The difference between the NPS and the NCEL Vane Shear vane rotation angles at failure
Figure 23: Typical continuous vane shear records produced by the NPS Vane Shear Apparatus.
appears to be due to the bending of a flexible reed used by the NCEL apparatus; therefore, it does not indicate the true angle of failure.
IV. DEEP SEA SEDIMENT CORE INVESTIGATION

A. LOCATION

The deep sea sediment cores were taken aboard the USNS BARTLETT (T-AGOR-13) on 24 April 1970, in the vicinity of Guide Seamount. The core locations are shown on Fig. 24 and are also listed in Table 1. Figure 25, a precision fathometer profile of the ocean bottom depicting the seamount, was made during the coring program. The original intent was to position the BARTLETT northwest of the seamount, just beyond the 1700 fathom curve; and take deep sea cores as the ship drifted with the wind and currents across the seamount, at the same time obtaining a fathometer profile of the coring area. However, as is shown on Fig. 24 and Fig. 25, the coring track was not a straight line across the seamount as a result of the sea conditions. After core 4H was obtained, a two mile northeasterly transit was made in order to reposition over the summit of the seamount. The ship was again allowed to drift, this time holding a different heading in order to move in a more southeasterly direction.

B. CORES

The coring was done with a Ewing gravity corer with a ten foot long, 2-1/2 inch diameter core barrel incorporating a plastic liner and a brass core retainer with a 450 pound driving weight. After the cores were brought on board ship, the plastic liners were removed from the core barrel, capped, taped with plastic electrical tape, and stored in an upright position in a large barrel made from two 55 gallon oil drums. The barrel was filled with salt water so as to protect the cores from desiccation. The barrel and cores were then
Figure 24: The track and location of the cores taken in the vicinity of Guide Seamount
<table>
<thead>
<tr>
<th>CORE NUMBER</th>
<th>LOCATION</th>
<th>WATER DEPTH</th>
<th>CORE LENGTH</th>
<th>DATE COLLECTED</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>LATITUDE</td>
<td>LONGITUDE</td>
<td>FATHOMS</td>
<td>(inches)</td>
</tr>
<tr>
<td>1H</td>
<td>37° - 04.2'N</td>
<td>123° - 23.1'W</td>
<td>1760</td>
<td>3218</td>
</tr>
<tr>
<td>2H</td>
<td>37° - 02.7'N</td>
<td>123° - 23.5'W</td>
<td>1665</td>
<td>3042</td>
</tr>
<tr>
<td>3H</td>
<td>37° - 01.8'N</td>
<td>123° - 23.4'W</td>
<td>1570</td>
<td>2871</td>
</tr>
<tr>
<td>4H</td>
<td>37° - 00.2'N</td>
<td>123° - 24.0'W</td>
<td>1410</td>
<td>2578</td>
</tr>
<tr>
<td>No Core</td>
<td>37° - 01.7'N</td>
<td>123° - 21.4'W</td>
<td>1210</td>
<td>2213</td>
</tr>
<tr>
<td>No Core</td>
<td>37° - 01.3'N</td>
<td>123° - 21.1'W</td>
<td>1010</td>
<td>1847</td>
</tr>
<tr>
<td>5H</td>
<td>36° - 58.8'N</td>
<td>123° - 17.8'W</td>
<td>1385</td>
<td>2533</td>
</tr>
<tr>
<td>6H</td>
<td>36° - 58.4'N</td>
<td>123° - 17.2'W</td>
<td>1370</td>
<td>2505</td>
</tr>
<tr>
<td>7H</td>
<td>36° - 57.5'N</td>
<td>123° - 16.2'W</td>
<td>1390</td>
<td>2542</td>
</tr>
<tr>
<td>8H</td>
<td>36° - 57.0'N</td>
<td>123° - 15.6'W</td>
<td>1445</td>
<td>2642</td>
</tr>
<tr>
<td>9H</td>
<td>36° - 56.4'N</td>
<td>123° - 14.6'W</td>
<td>1495</td>
<td>2734</td>
</tr>
<tr>
<td>10H</td>
<td>36° - 55.4'N</td>
<td>123° - 13.4'W</td>
<td>1600</td>
<td>2924</td>
</tr>
</tbody>
</table>
Figure 25: The precision fathometer profile made during the coring evolution across the Guide Seamount
transported back to the laboratory and stored until the testing could be done.

As indicated by Fig. 24 and Fig. 25, no cores were obtained on or near the top of the seamount. In two attempts, when the corer was brought on board ship, the cutter was badly scarred and chipped. The wire was also fouled around the fins of the corer and appeared as if it had rebounded on hitting the bottom. This failure to obtain cores in this area is probably because of the igneous rocks atop this volcanic seamount (Uchupi and Emery, 1961).

C. ENGINEERING PROPERTIES

A brief description of the engineering properties studied in this investigation are presented in Appendix B. The significance of the terms is considered in standard texts on soil mechanics. The following properties of the Guide Seamount cores were examined:

a. Shear Strength
b. Remolded Strength
c. Sensitivity
d. Bulk Wet Density
e. Water Content
f. Specific Gravity Solids
g. Dry Density
h. Void Ratio
i. Porosity

D. LABORATORY TEST SEQUENCE

The laboratory examination of the cores from the Guide Seamount region was conducted on three inch test interval sections. The sequence of testing was performed in the following manner, after the
NPS Vane Shear Apparatus had been calibrated by using the procedure previously outlined. A core was selected and the number, length, location and any other pertinent remarks were noted. Using the NPS Vane Shear Apparatus, the vane shear test was conducted with the following procedure: A vane size and recorder setting was determined with the aid of Appendix E by first estimating the approximate shear strength of the sample to be tested. The sample was then placed in the core holding bracket shown in Fig. 3 and the vane was lowered into the sample so that the top of the vane was 3/4 inch below the sample surface. The top and bottom lock-nuts shown in Fig. 14 were secured against the guide bracket. The strip chart recorder zero was then adjusted to a convenient reference line and the recorder advance was set at the desired speed. The advance of one inch per minute correlated well with the six degrees of vane rotation per minute provided by the vane shear motor and was therefore used in most cases. The vane shear motor was started when a reference line on the chart paper passed under the pen. The test was continued until a definite peak occurred on the recorder record, which was normally between 15 and 24 degrees of vane rotation. The shear strength in pounds per square inch was obtained from Appendix E by entering with the peak value of the record in millivolts and the size of the vane used. The shear strength may also be calculated by using the shear strength equation. Upon completion of the vane shear test, a small stainless steel sleeve of known weight and volume (Fig. 26) was inserted into an undisturbed portion of the sample to obtain a specimen for the bulk wet density determination. The sleeve with its sample was removed and weighed, with the results recorded on the appropriate
Figure 26: Detail drawing of the stainless steel sleeves manufactured for bulk wet density determination
worksheet, an example of which is shown in Fig. 27.

After the core was removed from the core holding bracket, a three inch section of the core liner was cut from the core by using a soldering gun, as illustrated by Fig. 28. This procedure is described in detail by Smith and Nunes (1963). The sediment portion of the core was cut with a piano wire blade coping saw, also shown in Fig. 28, and the remainder of the core was again sealed.

The water content was obtained by taking approximately a 25 gram sample from the top of each section tested and placing it in a small container of known weight. The container and the sample were then weighed immediately and placed in an oven held at a constant temperature of 110 degrees centigrade. After a 24 hour period, the sample container was removed from the oven, allowed to cool approximately two minutes, and then reweighed. The water content was calculated from the ratio of the weight of the water to the dry weight of the sample.

After removal of the water content sample, the remainder of the section was extruded into a plastic bowl where it was cut lengthwise with a small spatula to note the lithology. The sample was then remolded with the small spatula for approximately one minute and placed into a three inch core liner section with a bottom cap. The remolded strength was then determined in the same manner as the original by using the vane shear apparatus.

E. CALCULATIONS

The example worksheet shown in Fig. 27 exhibits the calculations performed for each sample tested. The results were then transferred to the core summary sheets in Appendix D. This data was then examined and evaluated with the aid of Appendix C and the various figures and tables included in this report.
**Figure 27:** The type of worksheet that was used in the data reduction of the Guide Seamount cores.
Figure 28: A demonstration of the soldering gun technique for cutting plastic core liners
F. RESULTS

The sediments of each core consisted predominately of a grayish-green clayey mud with a uniform color designation of 5GY 3/2 (Geological Society of America, 1951). Cores 6H, 7H, 8H, and 9H exhibited areas and layers of sand size material which were considerably more gritty in comparison to the rest of the core. The majority of this grittiness was attributed to larger tests within the pelagic sediments, with only about ten percent attributed to true sand grains. The tests were of both calcareous and siliceous derivation, the most prevalent observed being Foraminifera, diatoms and radiolarians. Mica was observed as the main component of the small amount of terrigenous mud, with a very slight trace of olivine also being detected (R. S. Andrews, personal communication). These more sandy areas served as the only real contrasting difference between the cores taken from the two sides of the seamount. The coarser areas were slightly darker in color than was the mud; however, the significance of this color dissimilarity was not considered sufficient to warrant a different color designation. The sediment exhibited a definite hydrogen sulfide odor throughout, but, according to Smith and Hironaka (1964), such odors are usually not a significant characteristic feature and depend both on the period of storage prior to testing and the experience and judgment of the recording technician. There were no large shells visible to the unaided eye in any of the cores.

1. Shear Strength

It is well established that the shear strength of normally consolidated soils increases with depth. This vertical variation of strength or cohesion with depth, shown in Figs. 29, 30, and 31, was
Figure 29: The variation of vane shear strength with depth for Guide Seamount cores 1H through 4H.
Figure 30: The variation of vane shear strength with depth for Guide Seamount cores 5 H through 7 H
Figure 31: The variation of vane shear strength with depth for Guide Seamount cores 8 H through 10 H
found to be far less consistent than either the bulk wet density or water content patterns. This may be attributed to the deposition of layers of varying character. According to Inderbitzen (1969), there was no direct correlation between rates of strength increase or water content decrease and sedimentation rates in the recent marine sediments off Southern California. In that grain size and composition were not determined for these samples, the basic reason for the shear strength variability was not determined.

Sediment shear strength includes both friction and cohesion terms and is primarily dependent upon the bulk density or packing. Electrostatic attraction between particles at boundaries in contact generally accounts for cohesion. The void ratio of a sediment would be decreased by more closely packed particles and the tighter packed particles would increase the attraction and cohesion. According to Hough (1957), internal friction is a function of the interlocking of grains as well as their frictional resistance to sliding, and is directly related to the bulk density of a granular material. Inderbitzen (1969) demonstrated an inverse relationship between the void ratio and the log of shear strength. A random check of the core results also confirmed such an inverse relationship in the present investigation.

The shear strengths of the sediments from the Guide Seamount region were found to range from 0.082 to 0.990 pounds per square inch, as shown in Table 2, with an average at the sediment surface of about 0.19 increasing to about 0.5 pounds per square inch two to three feet below the surface. With the exception of core 1H and 6H, all cores exhibit strengths of less than 0.5 pounds per square inch within their uppermost foot.

The values of shear strength of each test interval for all ten cores were plotted as data points on Fig. 32. Although not well defined,
TABLE 2: The minimum and maximum values of vane shear strength measured in the Guide Seamount cores

VANE SHEAR STRENGTH (psi)

<table>
<thead>
<tr>
<th>CORE</th>
<th>REMOLDED MIN</th>
<th>REMOLDED MAX</th>
<th>ORIGINAL MIN</th>
<th>ORIGINAL MAX</th>
</tr>
</thead>
<tbody>
<tr>
<td>1H</td>
<td>0.089</td>
<td>0.375</td>
<td>0.311</td>
<td>0.990</td>
</tr>
<tr>
<td>2H</td>
<td>0.055</td>
<td>0.245</td>
<td>0.232</td>
<td>0.802</td>
</tr>
<tr>
<td>3H</td>
<td>0.041</td>
<td>0.147</td>
<td>0.123</td>
<td>0.649</td>
</tr>
<tr>
<td>4H</td>
<td>0.048</td>
<td>0.109</td>
<td>0.133</td>
<td>0.570</td>
</tr>
<tr>
<td>5H</td>
<td>0.055</td>
<td>0.181</td>
<td>0.181</td>
<td>0.684</td>
</tr>
<tr>
<td>6H</td>
<td>0.072</td>
<td>0.147</td>
<td>0.270</td>
<td>0.663</td>
</tr>
<tr>
<td>7H</td>
<td>0.048</td>
<td>0.127</td>
<td>0.154</td>
<td>0.600</td>
</tr>
<tr>
<td>8H</td>
<td>0.048</td>
<td>0.133</td>
<td>0.201</td>
<td>0.560</td>
</tr>
<tr>
<td>9H</td>
<td>0.068</td>
<td>0.155</td>
<td>0.178</td>
<td>0.529</td>
</tr>
<tr>
<td>10H</td>
<td>0.050</td>
<td>0.161</td>
<td>0.082</td>
<td>0.795</td>
</tr>
</tbody>
</table>
Figure 32: The shear strength test interval data points for the ten Guide Seamount cores plotted as a function of depth.
the distribution does have a pattern. Examining these data points and plotting the frequency of occurrence for their shear strengths, as in Fig. 33, a statistical range appears with the greatest number of shear strengths between 0.3 and 0.6 pounds per square inch. Examination of these individual shear strength ranges as in Fig. 34, 35, and 36, demonstrates a wide distribution over the entire depth of the cores. The 0.3 to 0.4 pounds per square inch shear strength range is slightly skewed. By superimposing these three diagrams, as in Fig. 37, an inconsistency is noted in the depth interval of 24 to 27 inches. Comparing this drop in data points with those in Fig. 32, it is seen that the shear strength drops off in this interval and then increases again with depth. The absence of strength in the interval just described can also be noted by examining the core summary diagrams of Appendix C. There is no significant increase in the water content or decrease in the bulk density that would account for this loss in strength. It is suggested that perhaps this feature is related to disturbance induced during the corer penetration and retrieval during the sampling process.

2. Sensitivity

In some instances, a means of estimating the possibility of sample disturbance is by a measurement of sensitivity. The greater the sensitivity, the larger the loss of strength in the completely disturbed or remolded condition. Since sensitivity is also a function of the compositional character of the sediments, the amount of disturbance in these cores could not be defined specifically because the composition of these samples was not determined. In general, the remolded strength of the cores tested in this investigation was approximately one-third
Figure 33: The frequency of occurrence of the shear strength test interval data points of the ten Guide Seamount cores
Figure 34: The frequency of occurrence of the shear strength test interval data points ranging from 0.3 - 0.4 psi of the ten Guide Seamount cores.

Figure 35: The frequency of occurrence of the shear strength test interval data points ranging from 0.4 - 0.5 psi of the ten Guide Seamount cores.
Figure 36: The frequency of occurrence of the shear strength test interval data points ranging from 0.5 - 0.6 psi of the ten Guide Seamount cores.

Figure 37: The superposition of Figures 34, 35, and 36, frequency of occurrence of the shear strength test interval data points ranging from 0.3 - 0.6 psi of the Guide Seamount cores.
the original strength. Using the classification system of Rosenquist (1953), the sensitivity therefore ranged from slightly insensitive to very sensitive soil. The range of sensitivity values for the cores tested are given in Table 3. Appendix D contains a complete listing of the sensitivity values computed.

3. **Bulk Wet Density**

The bulk wet density shows a slight linear increase with depth, as illustrated by Figs. 38, 39 and 40. The overall range was from 1.16 to 1.73 grams per cubic centimeter, as shown in Table 4. The minimum value was from the 0-3 inch interval of core 7H and the maximum value, as evidenced by Fig. 40 and Appendix C, was in the "sand" layer in the 48-51 inch interval of core 8H. It is also noted in Appendix C that all of the high bulk wet density areas were associated with "sand" regions and zones of low water content.

4. **Original Water Content**

The water content exhibits a general decrease with depth, as shown in Figs. 41, 42 and 43, with a few low values occurring in the regions of high bulk wet density. As previously noted, these areas were related to the coarser grained material. Zones of high water content at depth were associated with cracks or voids in the core, as evidenced by core 3H in Appendix C, and these areas also exhibited low shear strength values. The water content ranged from 55.6 to 251 percent, as shown in Table 5. The low water content value was from the coarser layer in the 48-51 inch interval of core 8H and the high value from the 0-3 inch interval of core 1H. All of the cores tested revealed a water content of 190 percent or greater in the uppermost three inches, as shown in Appendices C and D.
TABLE 3: The minimum and maximum sensitivity values calculated for Guide Seamount cores

<table>
<thead>
<tr>
<th>CORE</th>
<th>MIN</th>
<th>MAX</th>
</tr>
</thead>
<tbody>
<tr>
<td>1H</td>
<td>2.07</td>
<td>5.23</td>
</tr>
<tr>
<td>2H</td>
<td>2.96</td>
<td>5.32</td>
</tr>
<tr>
<td>3H</td>
<td>2.25</td>
<td>7.78</td>
</tr>
<tr>
<td>4H</td>
<td>2.19</td>
<td>7.95</td>
</tr>
<tr>
<td>5H</td>
<td>2.65</td>
<td>5.00</td>
</tr>
<tr>
<td>6H</td>
<td>2.93</td>
<td>6.02</td>
</tr>
<tr>
<td>7H</td>
<td>1.81</td>
<td>7.64</td>
</tr>
<tr>
<td>8H</td>
<td>2.91</td>
<td>6.91</td>
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<tr>
<td>9H</td>
<td>2.61</td>
<td>6.75</td>
</tr>
<tr>
<td>10H</td>
<td>1.65</td>
<td>6.05</td>
</tr>
</tbody>
</table>
Figure 38: The variation of bulk wet density with depth for Guide Seamount cores 1H through 4H
Figure 39: The variation of bulk wet density with depth for Guide Seamount cores 5 H through 7 H
Figure 40: The variation of bulk wet density with depth for Guide Seamount cores 8 H through 10 H
TABLE 4: The minimum and maximum values of bulk wet density determined for Guide Seamount cores

<table>
<thead>
<tr>
<th>CORE</th>
<th>MIN</th>
<th>MAX</th>
</tr>
</thead>
<tbody>
<tr>
<td>1H</td>
<td>1.23</td>
<td>1.38</td>
</tr>
<tr>
<td>2H</td>
<td>1.23</td>
<td>1.35</td>
</tr>
<tr>
<td>3H</td>
<td>1.25</td>
<td>1.33</td>
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<td>1.45</td>
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<td>5H</td>
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<td>1.39</td>
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<tr>
<td>6H</td>
<td>1.25</td>
<td>1.52</td>
</tr>
<tr>
<td>7H</td>
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<td>1.25</td>
<td>1.46</td>
</tr>
<tr>
<td>10H</td>
<td>1.21</td>
<td>1.34</td>
</tr>
</tbody>
</table>
Figure 41: The variation of original water content with depth for Guide Seamount cores 1 H through 4 H
Figure 42: The variation of original water content with depth for Guide Seamount cores 5 H through 7 H
Figure 43: The variation of original water content with depth for Guide Seamount cores 8H through 10H
TABLE 5: The minimum and maximum values of original water content determined for Guide Seamount cores

<table>
<thead>
<tr>
<th>CORE</th>
<th>MIN</th>
<th>MAX</th>
</tr>
</thead>
<tbody>
<tr>
<td>1H</td>
<td>130</td>
<td>251</td>
</tr>
<tr>
<td>2H</td>
<td>143</td>
<td>230</td>
</tr>
<tr>
<td>3H</td>
<td>156</td>
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<td>4H</td>
<td>97.7</td>
<td>205</td>
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<td>78.6</td>
<td>190</td>
</tr>
<tr>
<td>10H</td>
<td>150</td>
<td>212</td>
</tr>
</tbody>
</table>
5. Specific Gravity of Solids

The specific gravity of solids was obtained by using the relationship derived by Henderson (1970), the derivation of which is included in Appendix A. This relates the specific gravity of solids to bulk wet density and water content, and according to Henderson (1970), this technique provides values that compare favorably with those obtained by other methods. Tables 6 and 7 are comparison values of the specific gravity of solids for all the cores tested, and a large variation with depth as well as between cores is quite evident. The specific gravity of the materials are noted as ranging from 1.70 to 3.26, as shown in Table 8. The 1.70 value was from the 0-3 inch interval of core 7H where the lowest bulk wet density reading was obtained and is probably not valid, due to improper sampling. High specific gravity values are usually an indication of high carbonate content; however, this was not measured to verify these readings. In comparing Table 7 with the core summaries of Appendix C for cores 6H, 7H, 8H, and 9H, there appears to be no relationship between the higher specific gravity values and the granular regions in these cores.

6. Void Ratio

The void ratio is a measure of the denseness of a sediment and is therefore one of its most important characteristics. In that the void ratio is proportional to water content, as noted by Lambe (1951), void ratio-depth plots tend to image the water content results, as is shown within Appendix C. Most of the void ratio values, as depicted on Figs. 44, 45 and 46, fall within the colloidal clay range as described by Hough (1957). The lowest void ratio value of 1.62 was found in the coarser layer of the 48-51 inch interval of core 8H, as
TABLE 6: The calculated specific gravity values for Guide Seamount cores 1H through 5H

<table>
<thead>
<tr>
<th>INTERVAL</th>
<th>CORE 1H</th>
<th>CORE 2H</th>
<th>CORE 3H</th>
<th>CORE 4H</th>
<th>CORE 5H</th>
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TABLE 7: The calculated specific gravity values for Guide Seamount cores 6 H through 10 H

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TABLE 8: The minimum and maximum specific gravity values calculated for Guide Seamount cores

SPECIFIC GRAVITY OF SOLIDS

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<th>MAX</th>
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</thead>
<tbody>
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<tr>
<td>10H</td>
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</table>
Figure 44: The variation of void ratio with depth for Guide Seamount cores 1H through 4H
Figure 45: The variation of void ratio with depth for Guide Seamount cores 5 H through 7 H
Figure 46: The variation of void ratio with depth for Guide Seamount cores 8 H through 10 H
shown in Table 9, and the highest value of 7.32 was from the two areas of high water content in core 1H. As the void ratio increases the shear strength decreases, or, assuming a 100 percent saturation, as the amount of water in the sediment increases the shear strength decreases.

7. Porosity

The porosity is also a measure of the denseness of a sediment and is thus similar to the void ratio. The porosity can generally be expected to decrease with depth due to normal consolidation of the underlying material by its overburden pressure. Such a result was found in all of the cores tested. The porosity values are given in the core summary sheets of Appendix D, and their range is shown by Table 10.

8. Dry Density

The dry density, or more commonly, the unit dry weight, is a calculated quantity and was found to range from 0.35 to 1.11 grams per cubic centimeter, as shown in Table 11. The values fall within the range of colloidal clays, as described by Hough (1957). A complete listing of the dry density values is included in the core summary sheets of Appendix D.
**TABLE 9:** The minimum and maximum void ratio values calculated for Guide Seamount cores

<table>
<thead>
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<th>CORE</th>
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<td>------</td>
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</table>
TABLE 11: The minimum and maximum dry density values calculated for Guide Seamount cores

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<th>MAX</th>
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</thead>
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</tr>
</tbody>
</table>
V. AT-SEA EVALUATION

A. GENERAL

In that the present state-of-the-art precludes the obtaining of reliable shear strength measurements in-situ on the sea floor, the next best alternative would appear to be to evaluate the shear strength of deep sea cores at-sea as soon as they are brought on board ship. In the past, most cores were stored aboard ship until they could be transported to a shore-based laboratory for testing. While on board ship, the cores are subjected to ship motion and machinery vibrations, causing disturbance of the sample. Also, any further transportation of the cores will result in sample disturbance.

In some instances, it has proven necessary to know what the bottom characteristics are at a given site immediately. As a consequence, a reliable method of measuring the sediment shear strength aboard ship would be most valuable. The vane shear method of shear testing lends itself readily to at-sea measurements.

The NPS Vane Shear Apparatus was therefore tested for at-sea use on board the USNS DE STEIGUER (T-AGOR-12) during NPS Cruise 1201 71 of 13-16 August 1970. The device was tested on the 15th of August on a gravity core taken at 36°-33' North Latitude and 122°-30 West Longitude with a Ewing corer, using 500 pounds of weight. The water depth at the coring site was 2760 fathoms and the core consisted of a grayish-green clayey mud similar in consistency to the Guide Seamount cores.

B. TEST PROCEDURE

After removal from the core barrel, the liner was sectionalized into three inch increments and every other section was capped and taped
for transportation back to Monterey. Testing was done in the sediment laboratory at the Naval Postgraduate School.

For the at-sea tests, the NPS Vane Shear apparatus was set up on the port side of the main laboratory of the DE STEIGUER. The top three inches was then tested, using the apparatus mounted on the table stand. The ship's motion indicated that the wall mounted core holding bracket method would perform better since the device and the sample would function more as a unit. The remainder of the tests made on board ship were conducted in this latter configuration and utilized various vane sizes. It was found that the size of the vane appeared to have little or no effect on the results of the test other than requiring the use of different scale settings on the recorder. The tests at the laboratory were conducted in the same configuration as the at-sea tests. Various vane sizes were alternated so as to be consistent with the at-sea tests.

High frequency machinery vibrations produced no apparent effect on the operation of the vane shear apparatus. As evidenced by Fig. 47, the motion of the ship did appear as a superposition upon the torque curve as plotted by the recorder. However, in each case of the at-sea tests, the characteristic shape of the output curve was maintained. A line through the average output values results in a curve of torque to well within the accuracy of the test.

The variation in the signal output caused by the ship's motion is produced by a pendulum action of the device coupled with the motion of the sample within its container. Accelerations caused by the ship's motion tend to move the sample within the core liner because of the extremely soft nature of the sediment. This tends to put a lateral force
Figure 47: Typical continuous vane shear records produced by the NPS Vane Shear Apparatus in laboratory and at-sea tests.
on the vane, which in turn causes the transducer to send a signal in addition to the normal output signal from the vane rotation. The magnitude of the fluctuations superimposed on the output signal can be reduced by using a larger vane size which requires a less sensitive scale setting on the recorder.

C. RESULTS

As demonstrated by Fig. 48, the values of shear strength in the tests conducted at-sea and in the laboratory compared favorably. That shear strength varies considerably with depth is evidenced by the Guide Seamount cores. Figure 49 is a curve made by combining the data points from the at-sea and laboratory tests. The amount of variability with depth is approximately the same as for the Guide Seamount cores and tends to corroborate that the at-sea curve gives as accurate results of the shear strength as the cores tested under laboratory conditions.
Figure 48: The variation of vane shear strength with depth comparing the results of laboratory and at-sea tests
Figure 49: The variation of vane shear strength with depth of the combined laboratory and at-sea tests
VI. SUMMARY AND CONCLUSIONS

Ten deep ocean gravity cores were collected from the Guide Seamount region located approximately 70 miles west of the Central California coast. These were transported to the sediment laboratory at the Naval Postgraduate School in Monterey, where their engineering properties were examined at three inch intervals.

The shear strength of the sediment cores was determined with the NPS Vane Shear Apparatus and was found to vary in a range from 0.082 to 0.990 pounds per square inch. Pelagic sediment areas were found in the cores taken from the southeastern side of the seamount and these exhibited high bulk wet densities and low water contents. Shear strength measurements in these areas were consistent with the findings for the rest of the core. The shear strength was generally found to increase with depth; however, a decrease was noted in most cores around the 24-27 inch interval followed by an increase below this area.

The NPS Vane Shear Apparatus was tested at-sea and the measured results were compared to on-shore laboratory tests. The results demonstrated that the apparatus was as effective at-sea as when used in the laboratory on-shore.
VII. RECOMMENDATIONS FOR FURTHER RESEARCH

The following studies are considered worthy of future investigation within the general area of vane shear testing of marine sediments:

a. Determine the effects of variation in the rate of rotation of the vanes.

b. Compare test results using various sized vanes and a variation in the number of blades in a material of reproducible strength to determine the effects of these variables.

c. Determine the optimum size of a vane to be used within a core liner with minimum effect on the shear strength by the liner.

d. Determine to what degree the shear strength of a sediment is affected by ship motion when performing tests at-sea.

e. Contrast shear strengths obtained from fresh cores at sea with cores from the same area that have been stored for a considerable period of time.
APPENDIX A

DERIVATIONS OF FORMULAS

Relationship of Shear Strength to Vane Shear Torque

\[ s = \text{shearing resistance of the sediment, pounds per square inch} \]
\[ T = \text{total torque at failure, inch-pounds} \]
\[ T_1 = \text{torque resistance on the vertical cylindrical surface, inch-pounds} \]
\[ T_2 = \text{torque resistance on the horizontal top and bottom, assuming constant unit shear resistance, inch-pounds} \]
\[ r = \text{radius of cylindrical plug sheared by vane, inches} \]
\[ h = \text{height of vane, inches} \]

Torque = Force X distance
\[ T = T_1 + 2T_2 \]
\[ T = (2\pi rh)r + 2(2\pi s \int_0^r x^2 \, dx) \]
\[ T = 2\pi r^2 hs + \frac{4}{3} \pi r^3 s \]
\[ T = s(2\pi r^2 h + \frac{4}{3} \pi r^3) \]
\[ s = \frac{T}{(2\pi r^2 h + \frac{4}{3} \pi r^3)} \]

Derivation of an Equation Relating Specific Gravity, Bulk Wet Density, and Water Content

Water content is defined as the weight of water in a sample divided by the weight of solids in the sample, i.e.,
\[ WC = \frac{W_W}{W_S} \]
(1)
In that the weight of a material is equal to the volume of the material times its density, equation (1) becomes:

\[ WC = \frac{V_W \times D_W}{V_S \times D_S} \]  

(2)

If the numerator and denominator of (2) are divided by the density of water at four degrees centigrade, equation (2) becomes:

\[ WC = \frac{V_W \times \frac{D_W}{D_4}}{V_S \times \frac{D_S}{D_4}} \]  

(3)

The ratio \( D_W/D_4 \) is the specific gravity of water and can be assumed equal to one for this derivation. \( D_S/D_4 \) is the specific gravity of solids. Equation (3) then becomes:

\[ WC = \frac{V_W}{V_S \times D_S} \]  

(4)

Bulk wet density is defined as the weight of a wet sample divided by its volume:

\[ BWD = \frac{W_t}{V_t} \]  

(5)

Ignoring the dissolved salts, the total weight of water is the weight of solids plus the weight of water, and the total volume is the volume of solids plus the volume of water. Equation (5) becomes:

\[ BWD = \frac{W_W + W_S}{V_W + V_S} \]  

(6)

Again, using the definition that weight equals volume times density, assuming the specific gravity of water is one, and dividing
the numerator and denominator of equation (6) by the density of distilled water at 4°C Centigrade, equation (6) becomes:

\[
BWD = \frac{\frac{V_w + V_s \times G_s}{V_w + V_s}}{D_4}
\]  

The density of distilled water at four degrees centigrade is one gram per cubic centimeter, and equation (7) is simplified to:

\[
BWD = \frac{V_w + V_s \times G_s}{V_w + V_s}
\]  

Dividing the numerator and denominator of equation (8) by \(V_w\) results in:

\[
1 + \frac{G_s \times \frac{V_s}{V_w}}{1 + \frac{V_s}{V_w}}
\]  

Equation (4) may be directly substituted into equation (9) to obtain:

\[
BWD = \frac{1 + \frac{1}{W_C}}{1 + \frac{V_s}{V_w}}
\]  

Equation (4) may be rearranged in the form:

\[
\frac{V_s}{V_w} = \frac{1}{W_C \times G_s}
\]

and equation (11) may be substituted into equation (10) to form:

\[
BWD = \frac{1 + \frac{1}{W_C}}{1 + \frac{1}{W_C \times G_s}}
\]
Equation (12) may be solved for $G_s$ to obtain:

$$G_s = \frac{\text{BWD}}{1 + \text{WC} - (\text{BWD} \times \text{WC})}$$

Symbols used in this derivation are:

- WC = water content, percentage
- $W_w$ = weight of water, grams
- $W_s$ = weight of solids, grams
- $D_w$ = density of water, grams per cubic centimeter
- $D_s$ = density of solids, grams per cubic centimeter
- $D_4$ = density of distilled water at four degrees centigrade, grams per cubic centimeter
- $G_s$ = specific gravity of solids
- $V_w$ = volume of water, cubic centimeters
- $V_s$ = volume of solids, cubic centimeters
- BWD = bulk wet density, grams per cubic centimeter
- $W_t$ = total weight of solids and water, grams
- $V_t$ = total volume of solids and water, grams
APPENDIX B

DEFINITIONS

Bulk Wet Density - The weight per unit volume of an undisturbed sample at its original water content.

Dry Density - The weight of soil solids per unit of total volume of soil mass.

Porosity - The ratio of the volume of voids to the total volume of the soil mass, expressed as a percentage.

Remolded Strength - The strength of sediment after it has had its natural structure modified by manipulation.

Sensitivity - The effect of remolding on the consistency of a cohesive sediment, as given by the ratio of the original strength to the remolded strength.

Shear Strength - The maximum resistance of a sediment to shearing stresses.

Shear Stress - The force per unit area acting with the sediment mass.

Specific Gravity of Solids - The ratio of the weight in air of a given volume of distilled water at a temperature of four degrees centigrade. The relationship of specific gravity, bulk wet density and water content is given in Appendix A.

Unit Weight - The weight per unit volume of soil mass. Although it is recognized that density is defined as mass per unit volume, in the field of soil mechanics the term is frequently used in place of unit weight.

Vane Shear Test - A shear test in which thin radial vanes are forced into the sediment and the resistance to rotation is determined.

Void Ratio - The ratio of the volume of void space to the volume of solid particles in a given sediment mass.

Water Content - The ratio of the weight of water in a given soil mass to the weight of solid particles, expressed as a percentage.
This Appendix contains a summary illustrating the variation of bulk wet density, vane shear strength, original water content and void ratio with depth for the ten deep-sea sediment cores from the Guide Seamount region. The areas designated "sand" represent sand size material which was composed mainly of pelagic sediments and was considerably more gritty than the rest of the core. The symbols "R" and "O" represent remolded and original shear strength, respectively.
CORE NO: 7H

- **Bulk Wet Density (g/cm³)**
- **Vane Shear Strength (psi)**
- **Water Content (percent)**
- **Void Ratio**

**Depth (feet):** 0, 5, 10, 15, 20, 25, 30, 35, 40, 45, 50, 55, 60

**Sand Layer:**

**R:**

**O:**
APPENDIX D

CORE SUMMARY SHEETS

This Appendix contains the core summary sheets for ten deep-sea sediment cores from the Guide Seamount region.
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<th>BULK DRY</th>
<th>RECOVERED</th>
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<th>MIN WET</th>
<th>SPREAD</th>
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<th>DENSITY</th>
<th>POROSITY</th>
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**NOTES:** Uniform grayish green color -- smooth greasy texture -- no structural variations.
**VESSEL**: USNS BARTLETT  
**CRUISE NO**: 137006  
**STATION NO**: 13  
**CORE NO**: 2H

**COLLECTED BY**: Westfahl & Hermann  
**DATE COLLECTION**: 4/24/70

**GENERAL REGION COLLECTION**: Guide Seamount  
**LATITUDE**: 37° 02.7' N  
**LONGITUDE**: 123° 23.5' W

**WATER DEPTH**: 1665 ft  
**HOW OBTAINED**: Precision Bath  
**ACCURACY**: No Correction

**CORING TOOL MAKE**: Ewing  
**TYPE**: Gravity  
**LENGTH CORE BARREL**: 10 ft  
**WEIGHT**: 450 lbs.

**ESTIMATED PENETRATION**: 11 ft  
**LENGTH CORE RECOVERED**: 42 in  
**GROSS RECOVERY RATIO (%)**: 31.8

**MIN INSIDE DIA BARREL**: 2.34 in  
**MIN INSIDE DIA CUTTER**: 2.36 in  
**INSIDE CLEARANCE RATIO (%)**: -0.85

**MAX OUTSIDE DIA BARREL**: 2.77 in  
**MAX OUTSIDE DIA CUTTER**: 2.38 in  
**OUTSIDE CLEARANCE RATIO (%)**: 21.85

**GROSS SEDIMENT FACES**: Pelagic  
**ORIGIN**: Terrigenous  
**RELATION TO TOPOGRAPHY**: Near base of Seamount

**STRATIGRAPHIC SIGNIFICANCE**: None

**REMARKS - SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS**: Uniform grayish green color -- smooth greasy texture -- no structural variations

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**NOTES**: None
VESSEL: USNS BARTLETT
CRUISE NO.: 137006
STATION NO.: 13
CORE NO.: 3

COLLECTED BY: Westfall & Hermann
DATE COLLECTION: 4/24/70

GENERAL REGION COLLECTION: Guide Seamount
LATITUDE: 37° 01.8' N
LONGITUDE: 123° 23.4' W

WATER DEPTH: 1570 Fath 2871 Meters
HOW OBTAINED: Precision Fath
ACCUACY: No Correction

CORING TOOL MAKE: Ewing
TYPE: Gravity
LENGTH CORE BARREL: 10 Ft.
WEIGHT: 450 lbs.

ESTIMATED PENETRATION: 10 Ft.
LENGTH CORE RECOVERED: 44 In.
GROSS RECOVERY RATIO (%): 36.6

MIN INSIDE DIA BARREL: 2.34 In.
MIN INSIDE DIA CUTTER: 2.36 In.
INSIDE CLEARANCE RATIO (%): -0.85

MAX OUTSIDE DIA BARREL: 2.77 In.
MAX OUTSIDE DIA CUTTER: 2.38 In.
OUTSIDE CLEARANCE RATIO (%): 21.85

GROSS SEDIMENT FACES: Pelagic
ORIGIN: Terrigenous

RELATION TO TOPOGRAPHY: Slope of Seamount

STRATIGRAPHIC SIGNIFICANCE: None

REMARKS - SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS:
Uniform grayish green color -- smooth greasy texture -- cracks and voids around 33 inch section of core

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<th>WAGE SHEAR STRENGTH (psi)</th>
<th>REMOVED STRENGTH (psi)</th>
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<th>WATER CONTENT (%)</th>
<th>SPECIFIC GRAVITY (g/cm³)</th>
<th>DRY DENSITY (g/cm³)</th>
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STRATIGRAPHIC SIGNIFICANCE: None

REMARKS-SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS: Uniform grayish green color -- smooth greasy texture -- no structural variations

NOTES:
**REMARKS-SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS:**

Uniform grayish green color -- majority of core had smooth, greasy texture except for gritty areas around 38 and 58 inch section of core.
**VESSEL**: USNS Bartlett  
**CRUISE NO**: 137006  
**STATION NO**: 11  
**CORE NO**: 7H

**COLLECTED BY**: Westfahl & Hermann  
**Guide Seamount**  
**DATE COLLECTION**: 4/24/70

**GENERAL REGION COLLECTION**: Lat. 36 - 57.5, Long. 123 - 16.2

**WATER DEPTH**: 1390 Fath  
**2542 Meters**  
**HOW OBTAINED**: Precision Path  
**ACCUACY**: No Correction

**CORING TOOL MAKE**: Ewing  
**TYPE**: Gravity  
**LENGTH CORE BARREL**: 10 Ft.  
**WEIGHT**: 450 lbs.

**ESTIMATED PENETRATION**: 10 Ft.  
**LENGTH CORE RECOVERED**: 52-1/2 ft.  
**GROSS RECOVERY RATIO (%)**: 43.6

**MIN INSIDE DIA BARREL**: 2.77 In.  
**MIN INSIDE DIA CUTTER**: 2.76 In.  
**INSIDE CLEARANCE RATIO (%)**: 21.85

**MAX OUTSIDE DIA BARREL**: 2.77 In.  
**MAX OUTSIDE DIA CUTTER**: 2.76 In.  
**OUTSIDE CLEARANCE RATIO (%)**: 21.85

**GROSS SEDIMENT FACIES**: Pelagic  
**ORIGIN**: Terrigenous

**RELATION TO TOPOGRAPHY**: Flat area near Seamount

**STRATIGRAPHIC SIGNIFICANCE**: None

**REMARKS—SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS**: Uniform grayish green color — smooth greasy texture except for gritty area around 38 inch section of core

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**NOTES:**
**VESSEL**: USNS BARTLETT

**CRUISE NO**: 137006  
**STATION NO**: 13  
**CORE NO**: 6H

**COLLECTED BY**: Westfahl & Hermann  
**DATE COLLECTION**: 4/24/70

**GENERAL REGION COLLECTION**: Guide Seamount

**WATER DEPTH**: 1445 Fath 2642 Meters  
**HOW OBTAINED**: Precision Path  
**ACCURACY**: No Correction

**CORING TOOL MAKE**: Ewing  
**TYPE**: Gravity  
**LENGTH CORE BARREL**: 10 Ft.  
**WEIGHT**: 450 lbs.

**ESTIMATED PENETRATION**: 9 Ft.  
**LENGTH CORE RECOVERED**: 54 In.  
**GROSS RECOVERY RATIO (%)**: 50

**MIN INSIDE DIA BARREL**: 2.34 In.  
**MIN INSIDE DIA CUTTER**: 2.36 In.  
**INSIDE CLEARANCE RATIO (%)**: 0.85

**MAX OUTSIDE DIA BARREL**: 2.77 In.  
**MAX OUTSIDE DIA CUTTER**: 2.38 In.  
**OUTSIDE CLEARANCE RATIO (%)**: 21.85

**GROSS SEDIMENT FACIES**: Pelagic  
**ORIGIN**: Terrigenous

**RELATION TO TOPOGRAPHY**: Flat area near Seamount

**STRATIGRAPHIC SIGNIFICANCE**: None

**REMARKS - SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS**: Uniform grayish green color -- smooth greasy texture except for the gritty area around 49 inch section of core

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**NOTES**: 
# Vessel: USNS BARTLETT

**Cruise no:** 137006  **Station no:** 13  **Core no:** 97

**COLLECTED BY:** Westfahl & Herrmann  **DATE COLLECTION:** 4/24/70

**General Region Collection:** Galde Seamount  **Latitude:** 36 - 56.4 N  **Longitude:** 123 - 14.6 W

**Water depth:** 1495 Fath  **How obtained:** Precision Bath  **Accuracy:** No Correction

**Corning tool make:** Ewing  **Type:** Gravity  **Length core barrel:** 10 ft.  **Weight:** 450 lbs.

**Estimated inside diam:** 2.34 in.  **Min inside DIA:** 2.34 in.  **Min inside cutter:** 2.36 in.

**Max outside DIA:** 2.77 in.  **Max outside cutter:** 2.38 in.  **Inside clearance ratio:** -0.85

**Outside clearance ratio:** 21.85  **Area ratio:** 104

**Gross sediment facies:** Pelagic  **Origin:** Terrigenous

**Relation to Topography:** Slope near base of Seamount

**Stratigraphic significance:** None

**Remarks - significance of color, texture, structure variations:** Uniform grayish green color -- smooth greasy texture except around the 50 inch section of core which was gritty
VESSEU: USNS BARTLETT
CRUISE NO: 137006
STATION NO: 13
CORE NO: 10H

COLLECTED BY: Westfahl & Hermann
DATE COLLECTION: 4/24/70

GENERAL REGION COLLECTION: Guide Seamount
LATITUDE: 36° 55.4' N
LONGITUDE: 123° 13.4' W
WATER DEPTH: 1600 fathoms
STATION NO: 13
CORE NO: 10H

COLLECTED BY: Westfahl & Hermann
DATE COLLECTED: 4/24/70

WATER DEPTH: 1600 fathoms 2924 meters
HOW OBTAINED: Precision Path
ACCURACY: No Correction

CORING TOOL MAKE: Ewing TYPE: Gravity LENGTH CORE BARREL: 10 ft. WEIGHT: 450 lbs.

ESTIMATED PENETRATION: 10 ft.
LENGTH CORE RECOVERED: 4 ft.
GROSS RECOVERY RATIO (%): 40

MIN INSIDE DIA BARREL: 2.34 in.
MIN INSIDE DIA CUTTER: 2.36 in.
MAX OUTSIDE DIA BARREL: 2.37 in.
MAX OUTSIDE DIA CUTTER: 2.38 in.
INSIDE CLEARANCE RATIO (%): -0.85
OUTSIDE CLEARANCE RATIO (%): 2.15

GROSS SEDIMENT FACIES: Pelagic ORIGIN: Terrigenous

RELATION TO TOPOGRAPHY: Base of Seamount

STRATIGRAPHIC SIGNIFICANCE: None

REMOKS-SIGNIFICANCE OF COLOR, TEXTURE, STRUCTURE VARIATIONS:
Uniform grayish green color -- smooth greasy texture -- no structural variations

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NOTES:
APPENDIX E

TABLE OF MILLIVOLTS - SHEAR STRENGTH

This Appendix contains a computer program and the generated table of millivolts - shear strength in pounds per square inch for various sized vanes used with the NPS Vane Shear Apparatus.
Computer Program for Generating a Table of Millivolts -- Shear Strength
in Pounds per Square Inch for Various Size Vanes

REAL*8 VOLTS
VANE1=0.45813
VANE2=0.61780
VANE3=1.30899
VANE4=2.09438
VANE5=2.87978
VOLTS=0.0
DO 1000 I=1,5001
       VOLTS=VOLTS + 0.10000 + 1.E-08
       IF(I.EQ.2500) GO TO 500
  600      SHEAR1=VOLTS/(64.0*VANE1)
       SHEAR2=VOLTS/(64.0*VANE2)
       SHEAR3=VOLTS/(64.0*VANE3)
       SHEAR4=VOLTS/(64.0*VANE4)
       SHEAR5=VOLTS/(64.0*VANE5)
       IF(I.EQ.1) GO TO 950
       J=I/50*50
       IF(J.EQ.1) GO TO 950
  900      WRITE (6,1001) VOLTS,SHEAR1,SHEAR2,SHEAR3,SHEAR4,SHEAR5
  1001      FORMAT (10X,1F10.1,5F10.3)
       GO TO 1000
  950      WRITE (6,1100)
  1100      FORMAT ('1','//////')
  1500      WRITE (6,2001)
  2001      FORMAT (14X,'RECORDER',3X,'VANE',6X,'VANE',6X,'VANE',6X,'VANE',5X,
         9'VANE')
       WRITE (6,2002)
  2002      FORMAT (14X,'READING',3X,'DIA=.5''',3X,'DIA=.5''',3X,'DIA=1''',4X,'DIA
         9=1''',3X,'DIA=1''')
       WRITE (6,2003)
  2003      FORMAT (24X,'HT = 1''',3X,'HT -.5''',3X,'HT=.5''',4X,'HT =1''',3X,'HT
         9=1.5''',/)
WRITE (6,2004)
2004 FORMAT(15X,'MILLI-',3X,'SHEAR',5X,'SHEAR',5X,'SHEAR',5X,'SHEAR',4X
9,'SHEAR')
WRITE (6,2005)
2005 FORMAT(15X,'VOLTS',3X,'STRENGTH',2X,'STRENGTH',2X,'STRENGTH',2X,
9,'STRENGTH',1X,'STRENGTH')
WRITE (5,2006)
2006 FORMAT (24X, '(PSI)',5X,'(PSI)',5X,'(PSI)',5X,'(PSI)',5X,'(PSI)',4X,'(PSI)',
9//)
GO TO 900
500 VOLTS=250.0000
GO TO 600
1000 CONTINUE
STOP
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BIBLIOGRAPHY


Smith, R. J., and Nunes, L., (1963), "Heated Element Sectioning of Plastic Core Liners and Core Barrels," Technical Note N-551, U. S. Naval Civil Engineering Laboratory, Port Hueneme, California.


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A sophisticated vane shear apparatus for determining the shear strength of deep ocean sediment cores was modified so as to be portable and versatile for use in a laboratory and on board a ship. The apparatus utilizes a torque transducer that is insensitive to temperature changes or orientation and capable of measuring torque over the entire range of shear strength encountered in marine sediments. Shear strength measurements can be made with a minimum disturbance to the sediment sample by testing directly in the core liner. The apparatus was used to determine shear strength of ten deep ocean cores from the Guide Seamount region, located about 70 miles west of the central California coast. The study also included the determination of other engineering properties of the sediment cores.
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Engineering properties of sediments in the vicinity of Guide Seamount.